SLIDING FAILURES AT FIVE LOCK AND DAM PROJECTS

Dam and Cofferdam Failures within the Ohio River Basin

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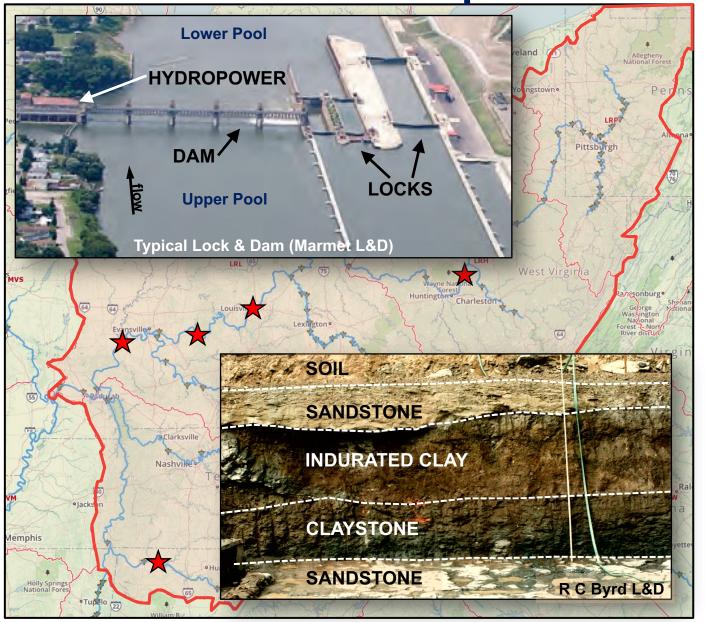
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Common Aspects of the Five Projects



- Located within the Ohio River Basin
- Lock and dam projects
- 1 dam and 4 cofferdams
- Founded on near-horizontal, interbedded, sedimentary rock
- 100's of feet of structure slid
- Failures from 1912 1971,
 listed in chronological order

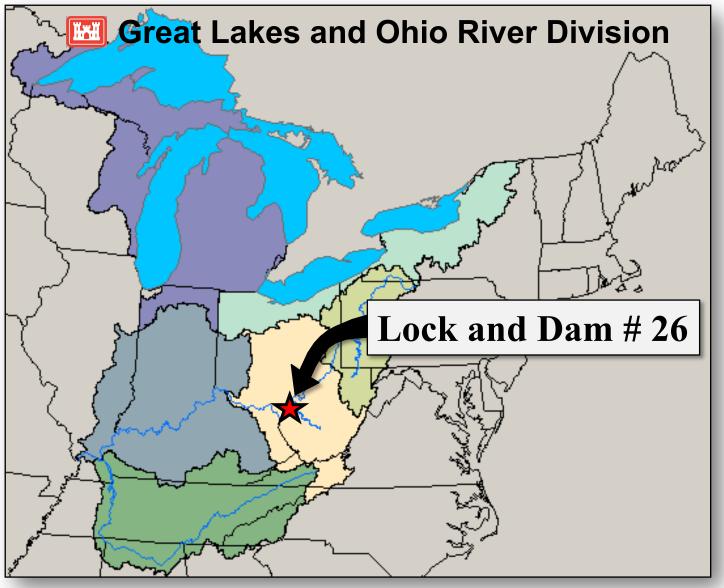


Sliding Failures at Five Lock and Dam Projects

OUTLINE

- 1. Ohio River Lock and Dam #26
- 2. Louisville and Portland Canal
- 3. Wheeler Lock and Dam
- 4. Cannelton Lock and Dam
- 5. J T Myers Lock and Dam

Ohio River Lock and Dam # 26 – General Information

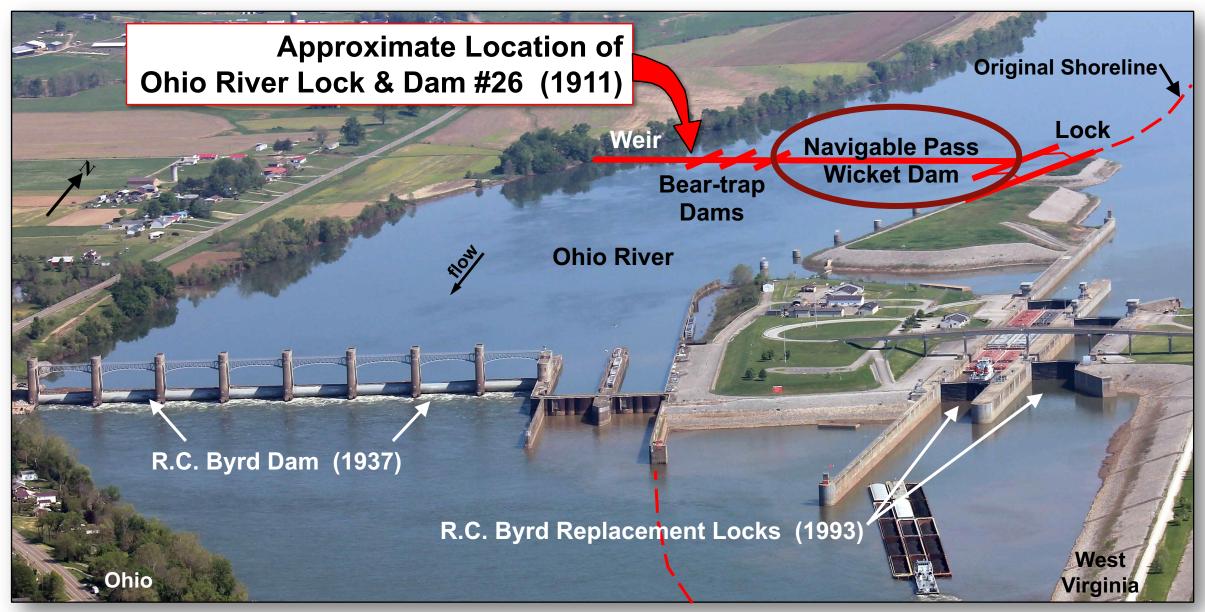


- Located on the Ohio River, at WV/OH boarder.
- Canalization of the Ohio River
 - > 49 locks & dams (now 19)
 - > 9' deep channel
 - > 600' x 110' lock chambers
 - Completed 1929
- Lock and Dam #26 Constructed in 1911
- Wicket Dam failed Aug 8, 1912



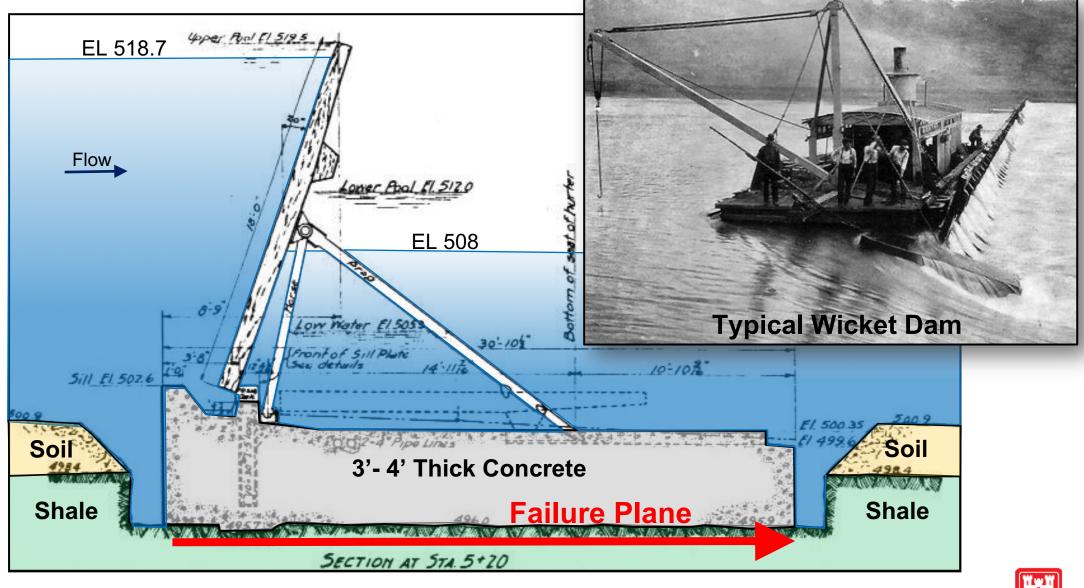


Ohio River Lock and Dam # 26 - Location



Aerial Photo – R.C. Byrd Locks and Dam

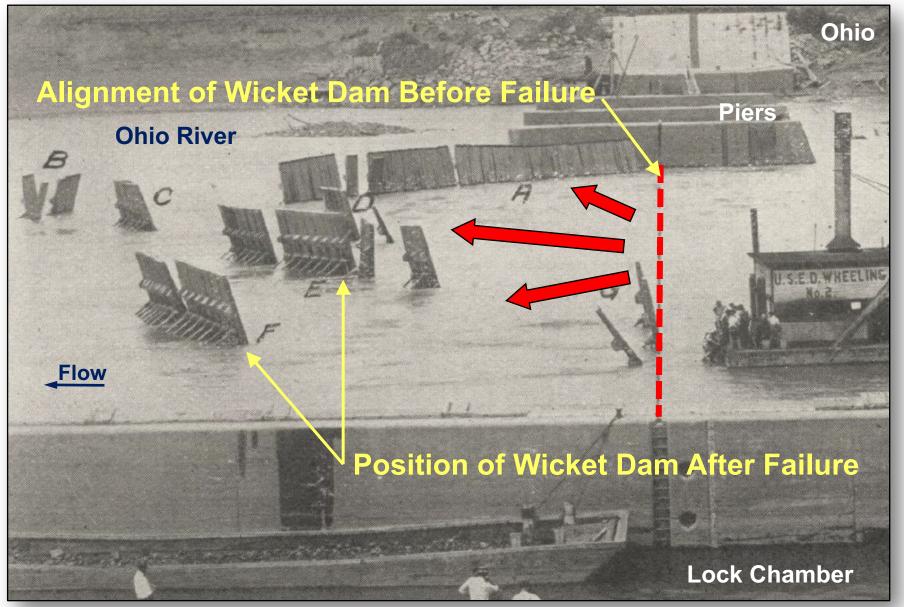
Ohio River Lock and Dam # 26 - Wicket Dam



Cross Section – Chanoine Wicket Dam – Ohio River Lock and Dam #26



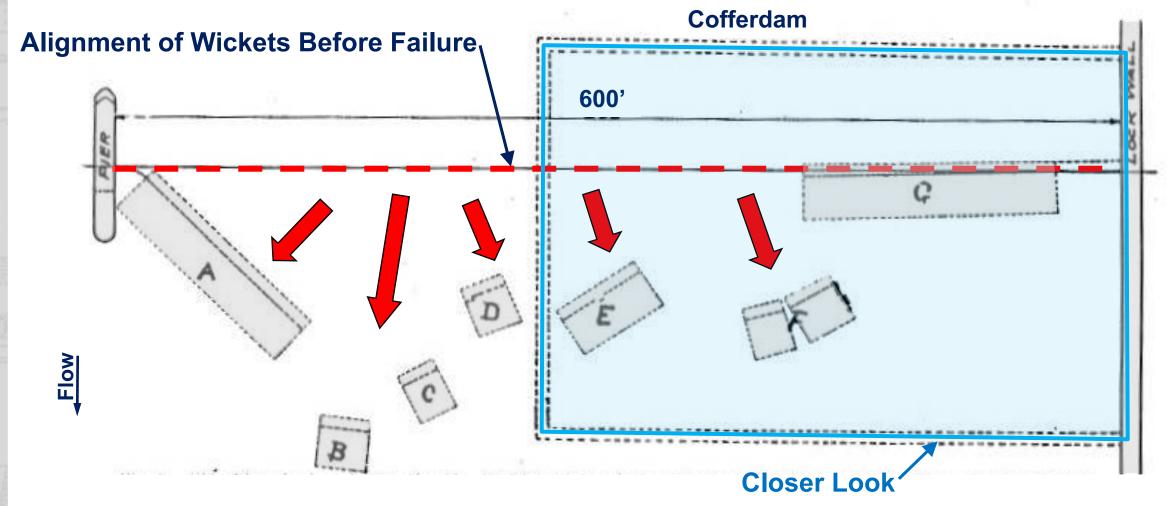
Ohio River Lock and Dam # 26 - Dam Failure



- Second time wickets were raised
- 10.7' head of water
- Failed on Aug 8, 1912 at 6:30 am
- Initiated in middle of 600' long dam
- Slid up to 157 feet downstream

Photo taken from West Virginia Side – After Failure

Ohio River Lock and Dam # 26 – Monolith Locations

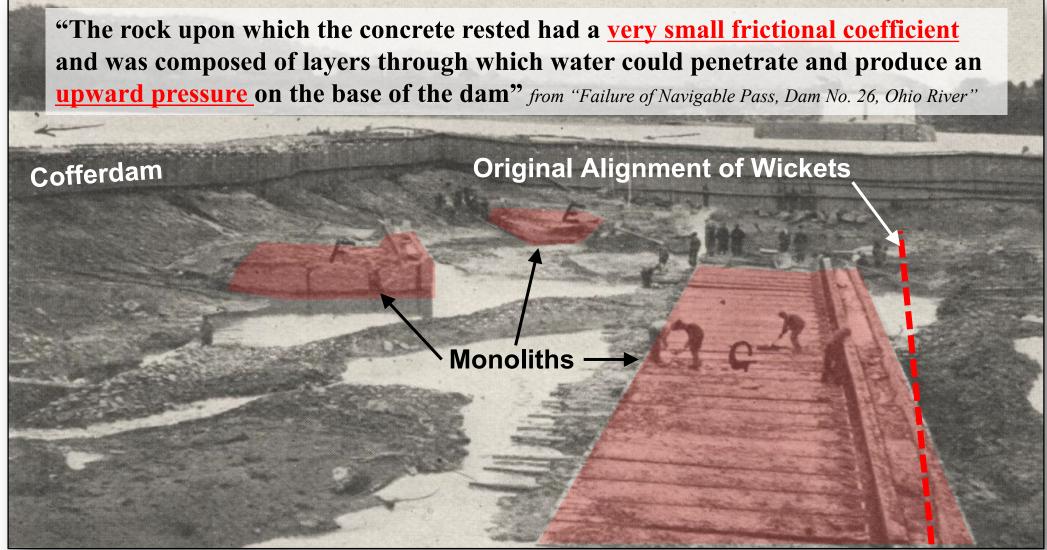


Plan View – Position of Navigable Pass Foundations After Failure





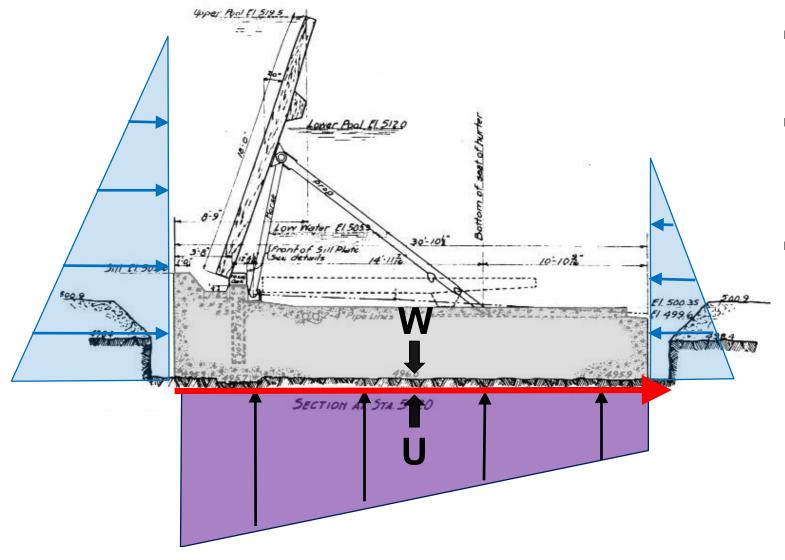
Ohio River Lock and Dam # 26 – Dewatered Monoliths







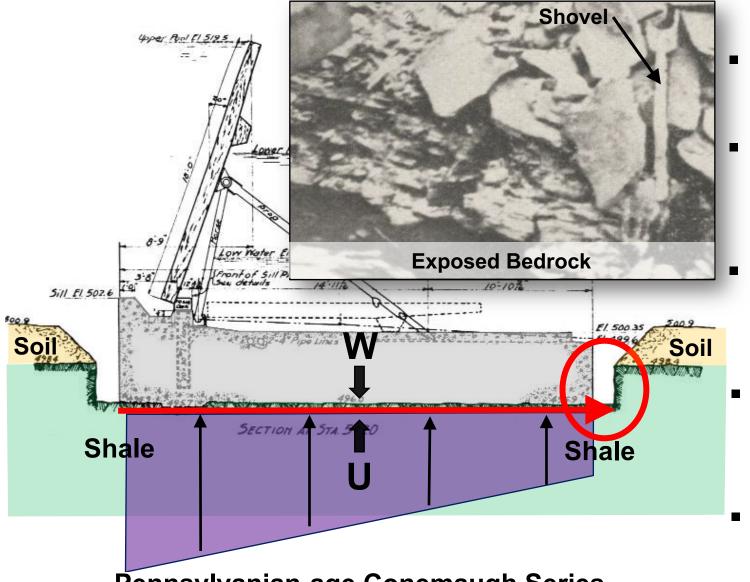
Ohio River Lock and Dam # 26 – Factors Leading to Failure



- Sliding plane was slightly below base.
- Original design: full uplift acting on 50% of foundation (typical for that era).
- Today, without drains, full uplift would be applied to 100% of foundation.



Ohio River Lock and Dam # 26 – Factors Leading to Failure



- Sliding plane was slightly below base.
- Original design: full uplift acting on 50% of foundation (typical for that era).
 - Today, without drains, full uplift would be applied to 100% of foundation.
- Sliding resistance of bedrock (shale) was overestimated
- No passive wedge

Pennsylvanian-age Conemaugh Series (likely a claystone $\phi = 27^{\circ}$)

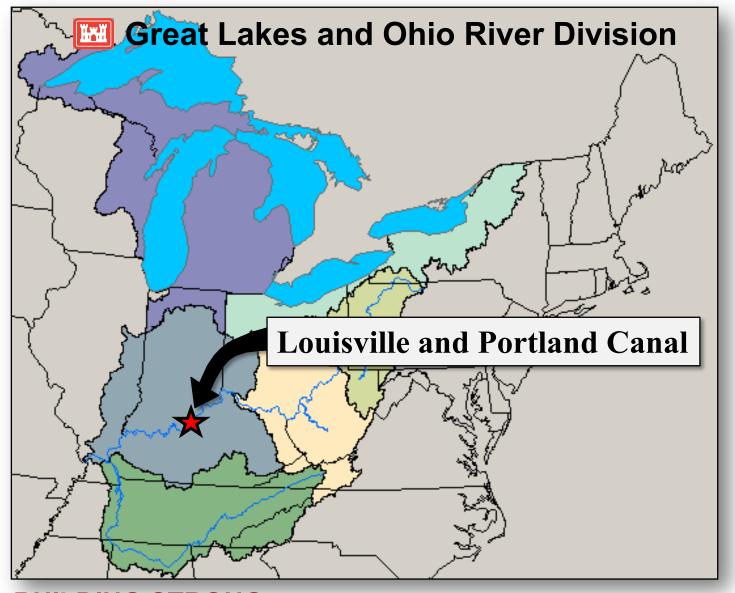


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Louisville and Portland Canal – General Information

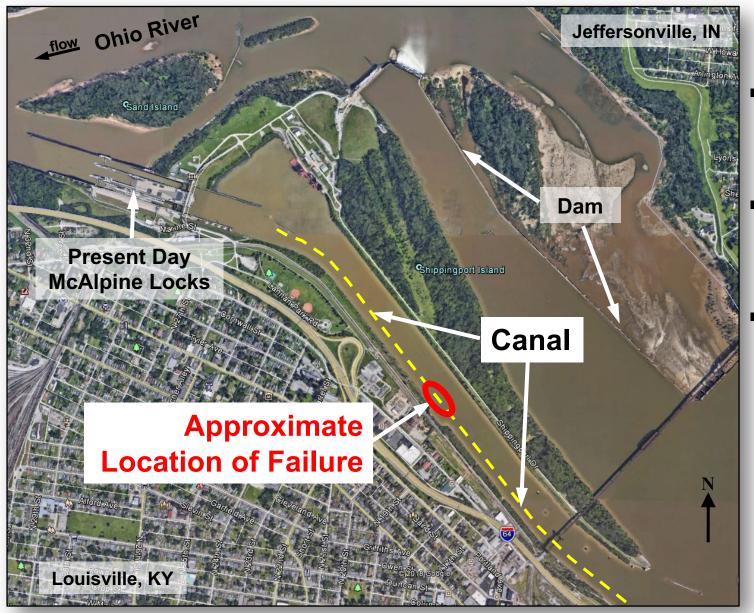


- Located on the Ohio River at Louisville, KY
- Canal widened in 1915
- Construction contractordesigned cofferdam failed Oct. 5, 1915



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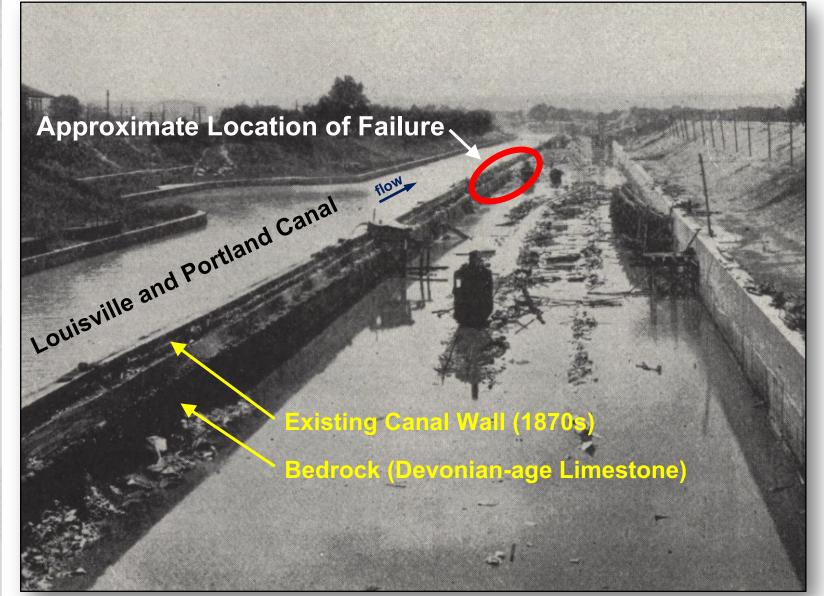
Louisville and Portland Canal – Location & History



- Falls of the Ohio, obstruction to navigation, 26' drop in 2 mi
- 1830's: original canal (50' wide)
 with series of three locks
- Widened canal and larger locks:
 - > 1870's
 - > 1910's 1920's
 - > 1960's
 - > 1990's



Louisville and Portland Canal – Dewatered Cofferdam



Construction Photo – Widening Canal

- Widening canal from 87' to 200'
- Construction contractor used the pre-existing canal wall (built 1870s) and underlying limestone as a cofferdam
- Cofferdam failed at 7:50am on Oct. 5, 1915
- One death



Louisville and Portland Canal – Cofferdam Failure

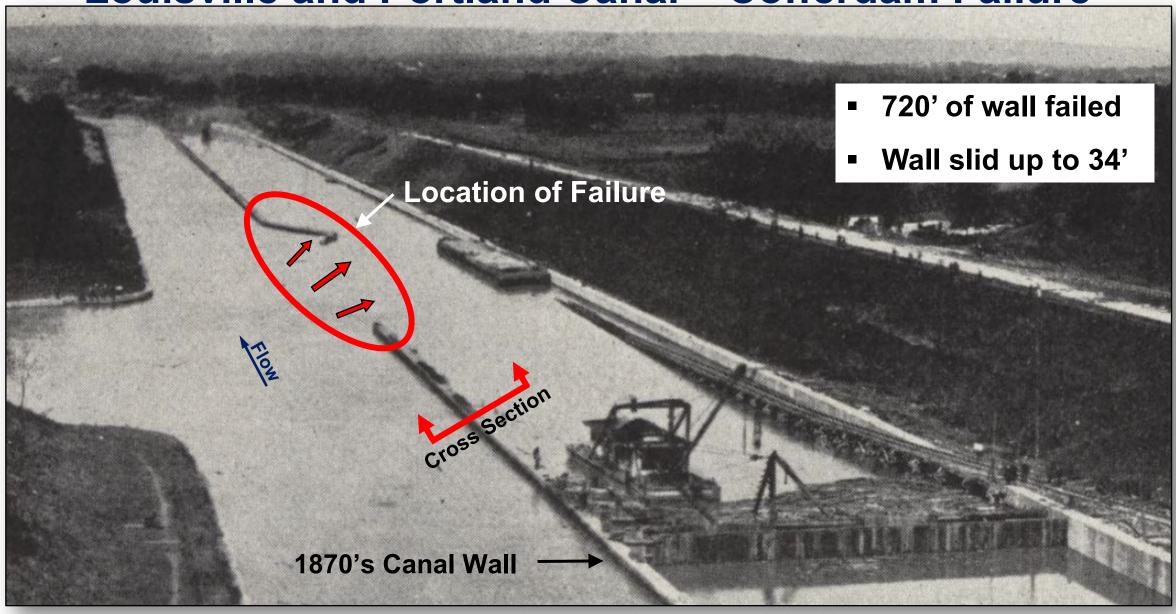
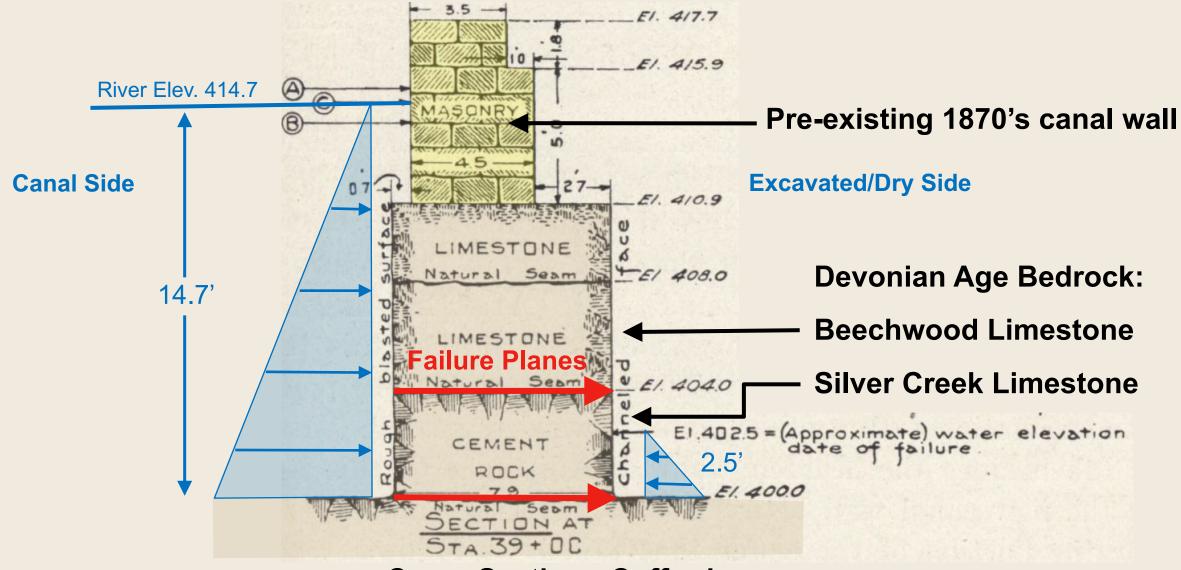


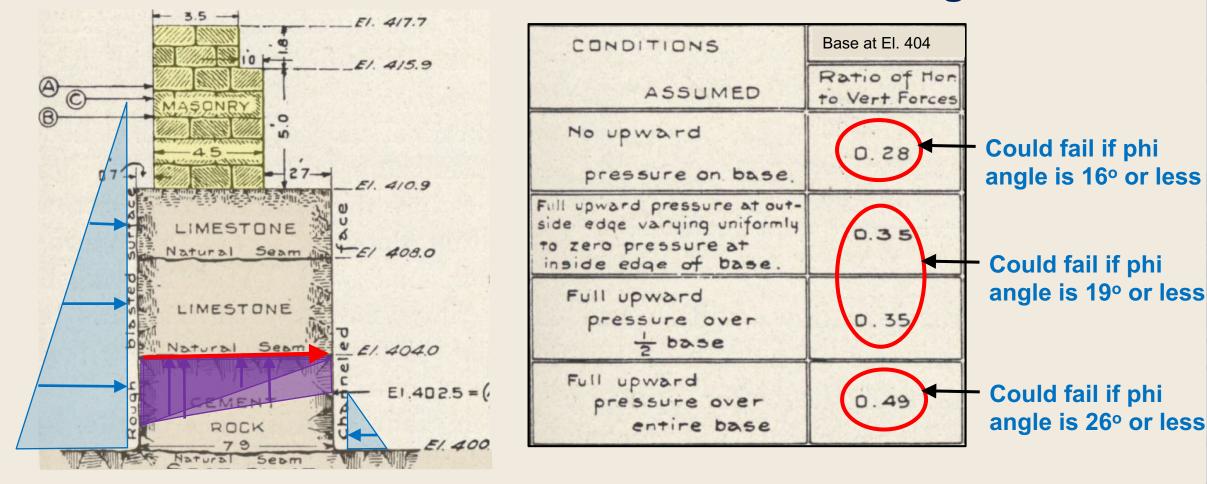
Photo – Location of Failed Section of Cofferdam

Louisville and Portland Canal – Failure Planes



Cross Section - Cofferdam

Louisville and Portland Canal – Factors leading to Failure



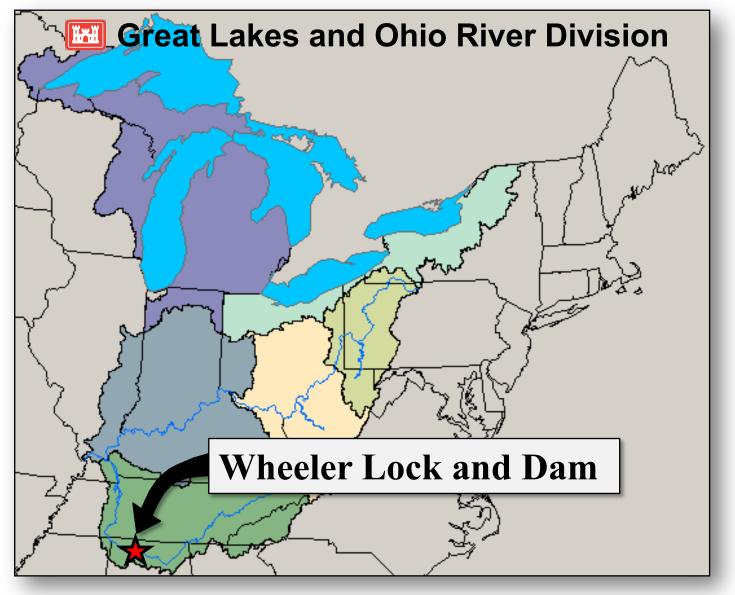
- Wedges were used to <u>stop seepage</u> from dry side (increased uplift pressure)
- Some borings had "silt or clay seam" at elev. 404 (lower phi angle)
- Blasting may have moved/shifted bedrock (post-peak rock strength)

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Wheeler Lock and Dam – General Information



- Located on the Tennessee
 River in Northern Alabama.
- Muscle Shoals obstruction
- Original lock constructed in 1934
- Additional 110' x 600' lock proposed 1960s, located adjacent to the original lock.
- Original lock wall was used as part of the cofferdam, failed in 1961.





Wheeler Lock and Dam – Location of New Lock

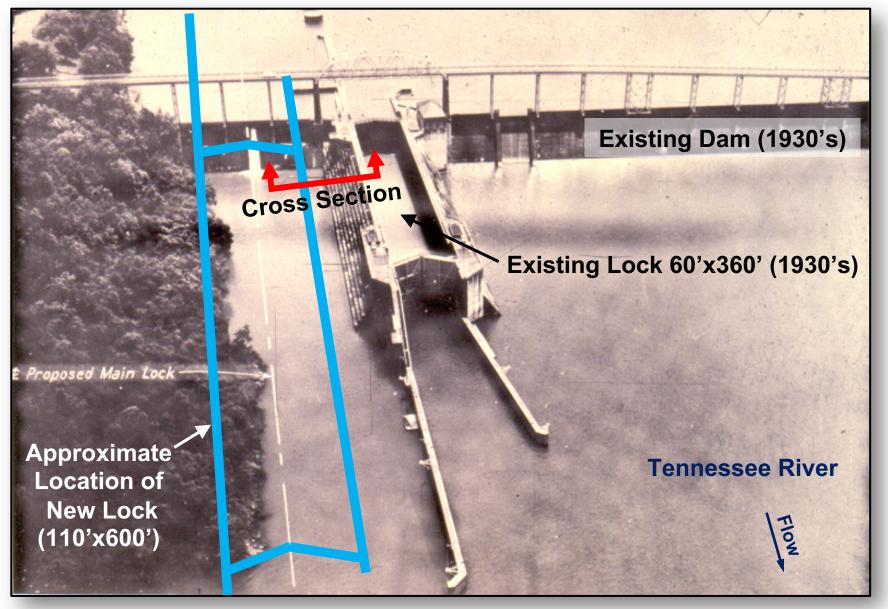
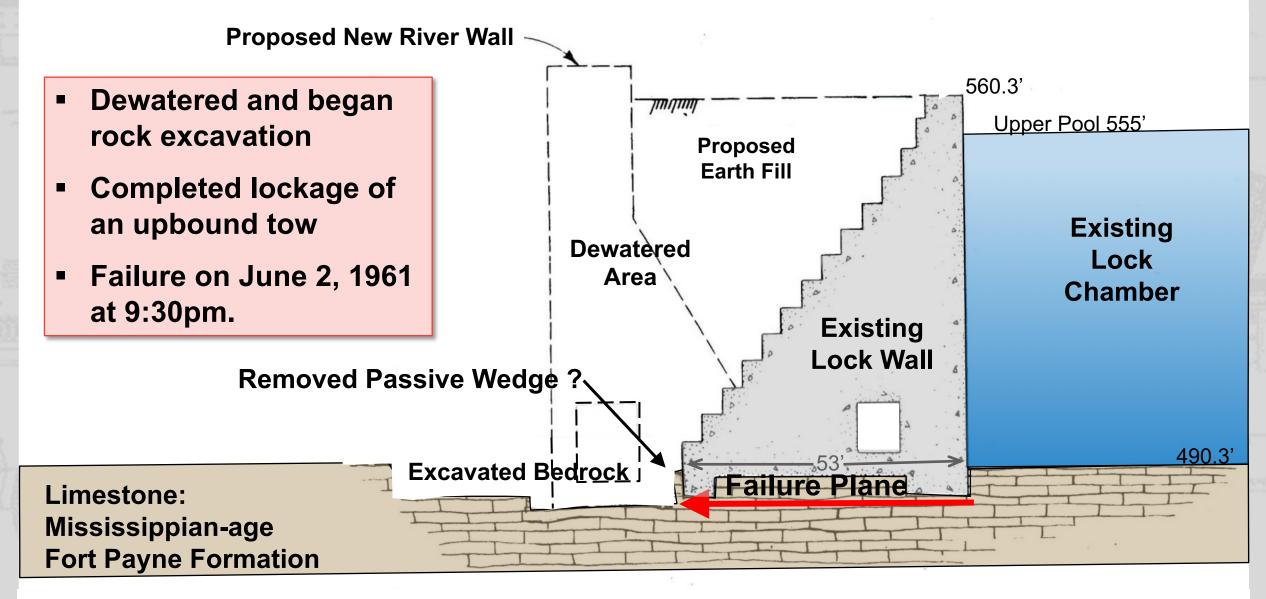


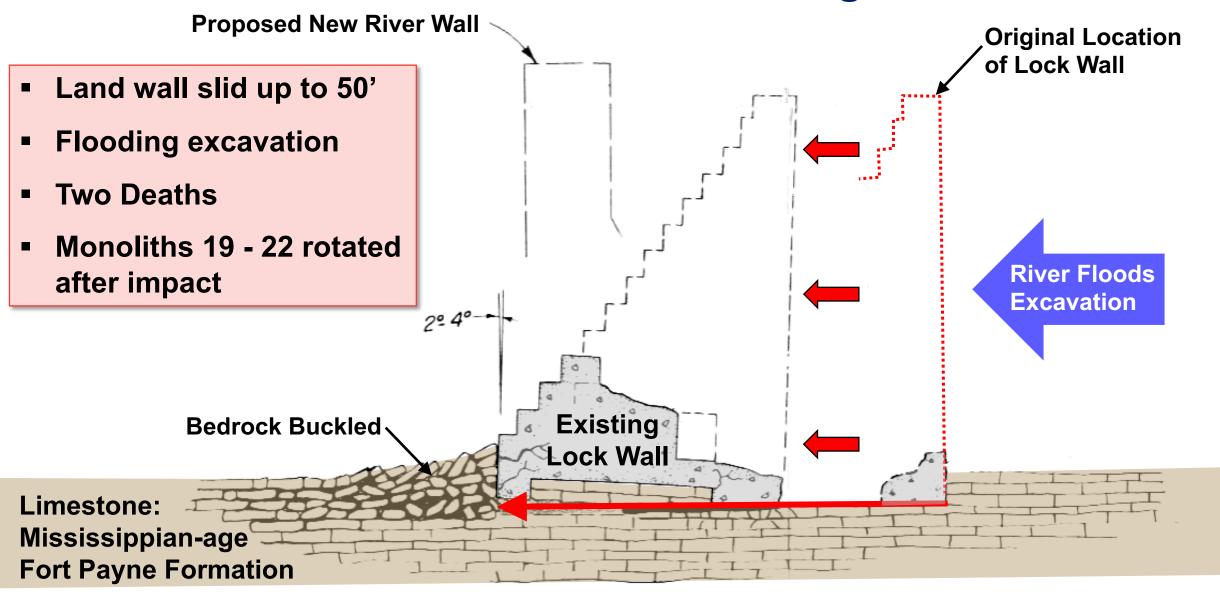
Photo Prior to Construction of Proposed New Lock

Wheeler Lock and Dam – Excavation for New Lock



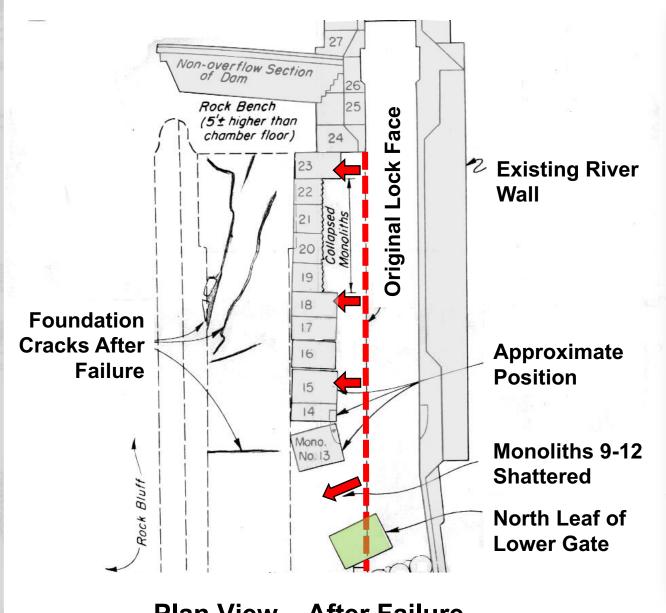
Cross Section – Lock Wall – Before Failure

Wheeler Lock and Dam – Sliding Failure



Cross Section – Lock Wall – After Failure

Wheeler Lock and Dam – Plan View and Photo After Failure



Original **Lock Face**

Aerial Photo – After Failure - Dewatered

Plan View – After Failure

Wheeler Lock and Dam – Photo After Failure

- Seepage noted during lockage
- Blast may have been concurrent with failure



Photo Viewed from Proposed New Lock After Failure of Original Lock

Wheeler Lock and Dam – Factors Leading to Failure



Photo Looking Downstream – After Failure

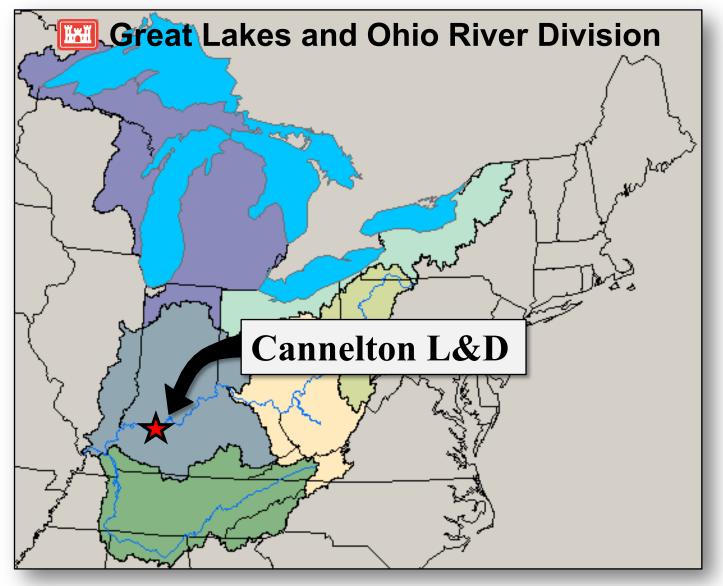
- Thick limestone foundation having a shale seam (0.5' thick), with an undetected thin clay seam (1/16 to 3/8" thick) at the base of the shale.
- Overestimated sliding resistance of bedrock
- Excavation "daylighted" clay seam, removing the passive wedge (blasting impact?)

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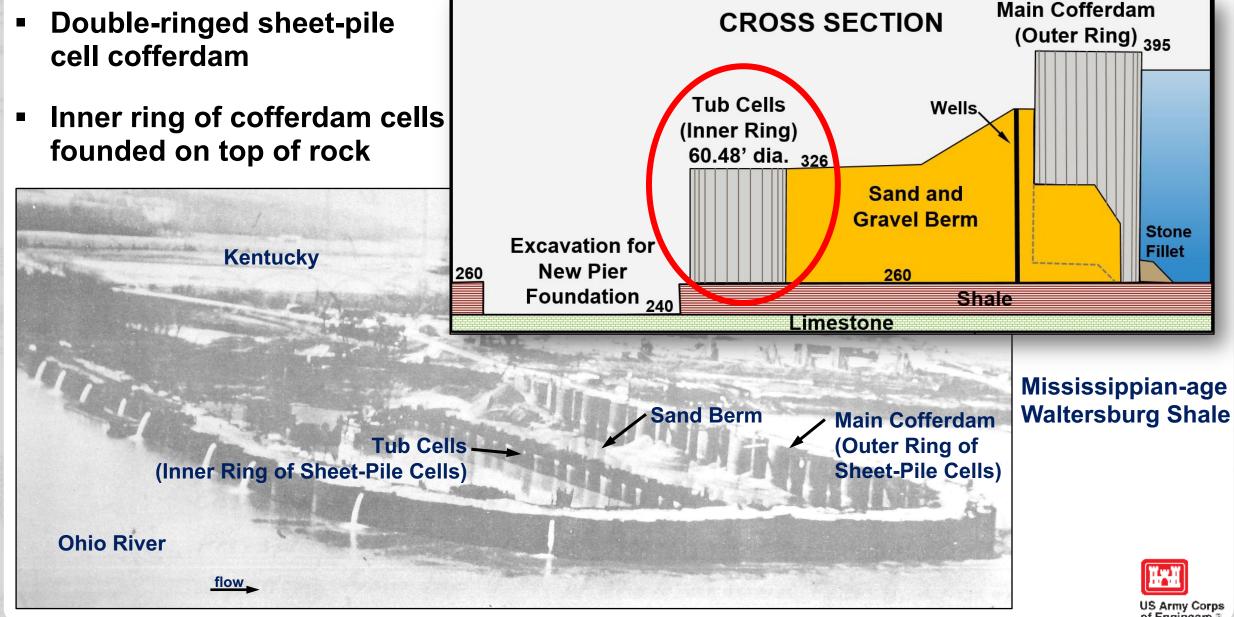
Cannelton Lock and Dam – General Information



- Located on the Ohio River at IN/KY boarder.
- Construction contractordesigned double-ringed sheet-pile cell cofferdam
- Excavation for the new dam pier foundations
- Progressive sliding failure, with distress indicators from Oct 19 to Nov 1, 1967 (14 days)

Cannelton Lock and Dam - Photo & Section of Cofferdam

cell cofferdam

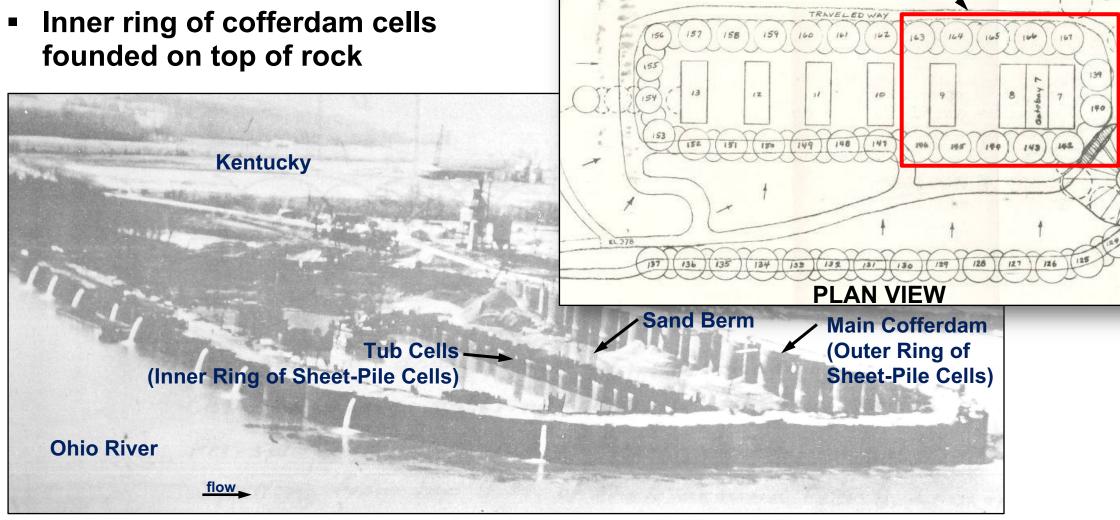




Cannelton Lock and Dam – Photo & Plan View of Cofferdam

Closer Look

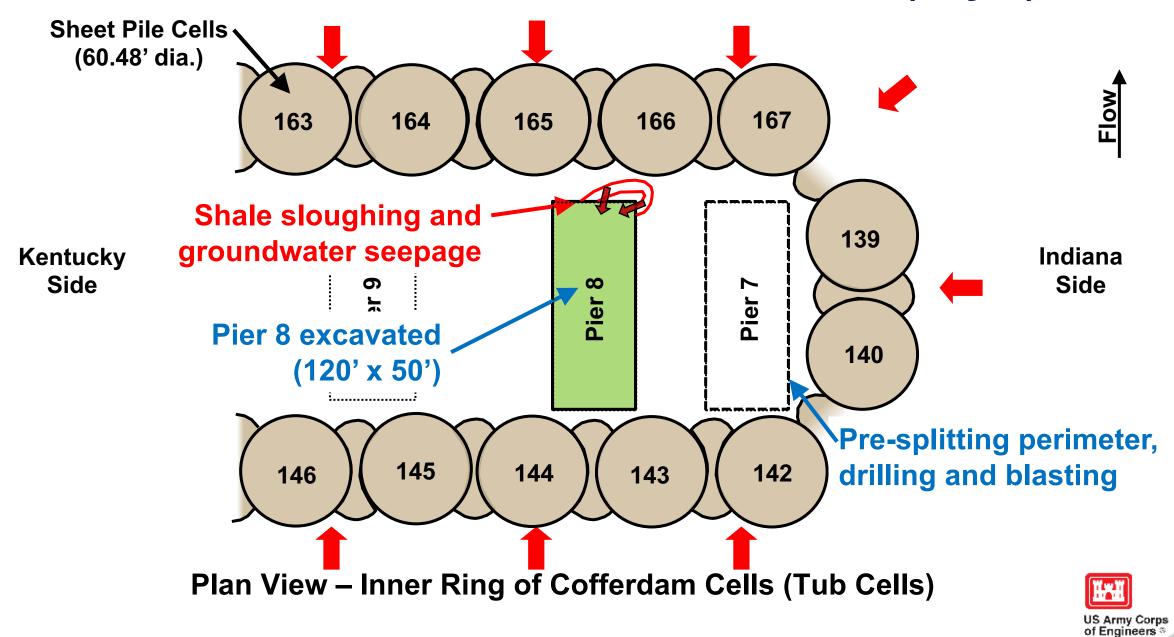
Double-ringed sheet-pile cell cofferdam





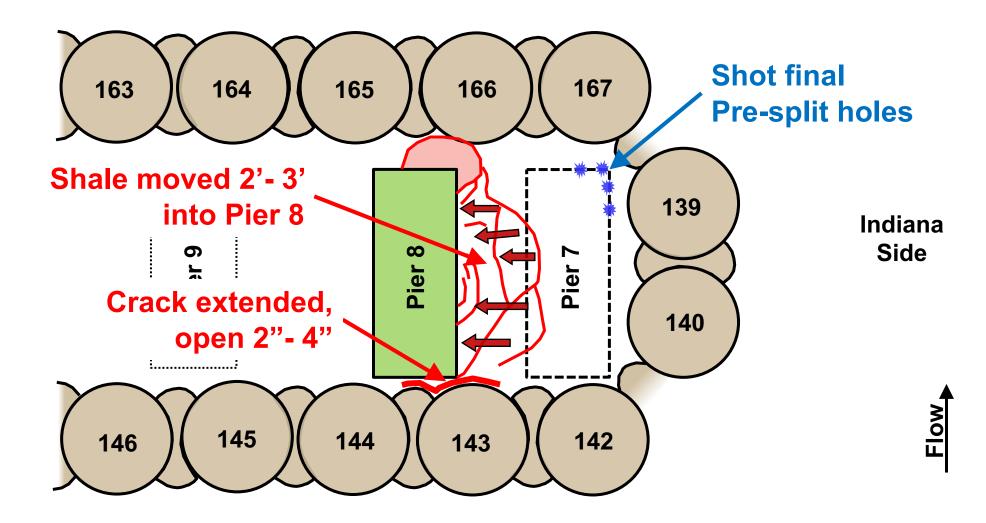
1"=100"

Cannelton Lock and Dam – Oct 19, 1967 (day 1)



Cannelton Lock and Dam – Oct 21, 1967 (day 3)

Kentucky Side



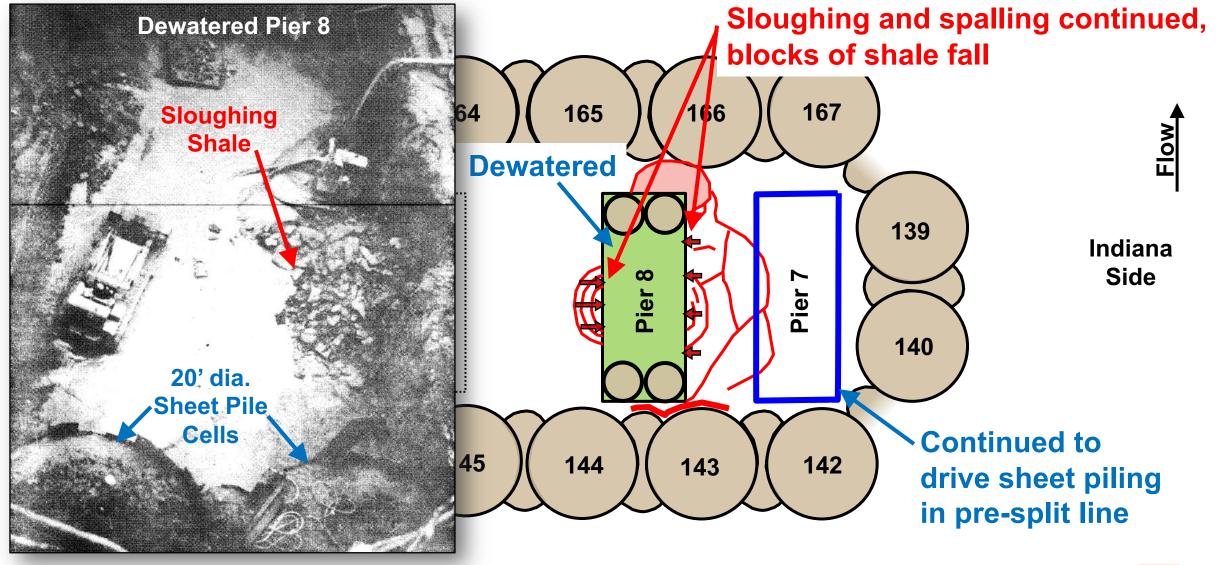
Plan View – Inner Ring of Cofferdam Cells (Tub Cells)



Cannelton Lock and Dam – Oct 27, 1967 (day 9) Pier 7 Sheet Piles 167 166 139 Indiana 20' dia Sheet Pile Cells Pier 7 Side Flooded Pier 8, 140 Placed 20' dia. cells **Driving sheet piling** 145 143 146 144 142 in pre-split line **Crack widening** Plan View - Inner Ring of Cofferdam Cells (Tub Cells)

US Army Corps of Engineers ®

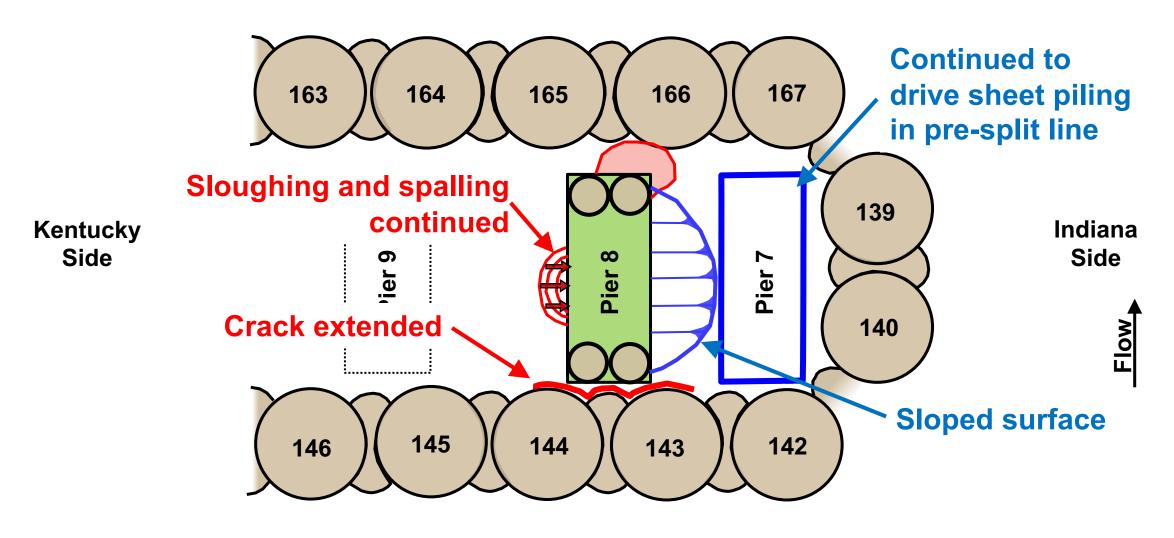
Cannelton Lock and Dam – Oct 31, 1967 (day 13 morning)



Plan View – Inner Ring of Cofferdam Cells (Tub Cells)



Cannelton Lock and Dam – Oct 31, 1967 (day 13 evening)

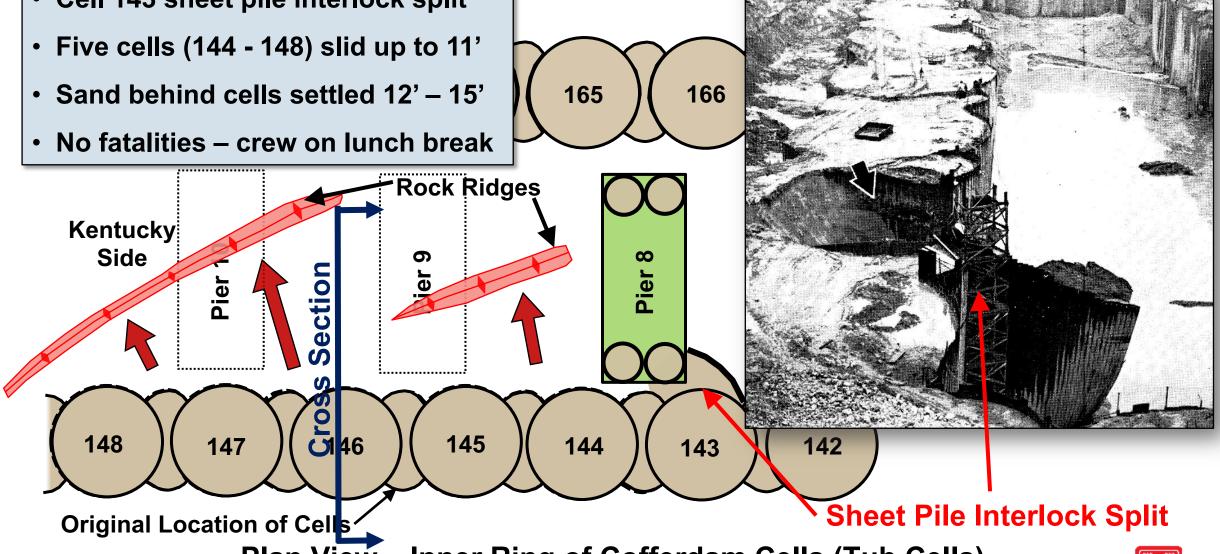


Plan View – Inner Ring of Cofferdam Cells (Tub Cells)



Cannelton Lock and Dam - Nov 1, 1967 @ 0430 hr (day 14)



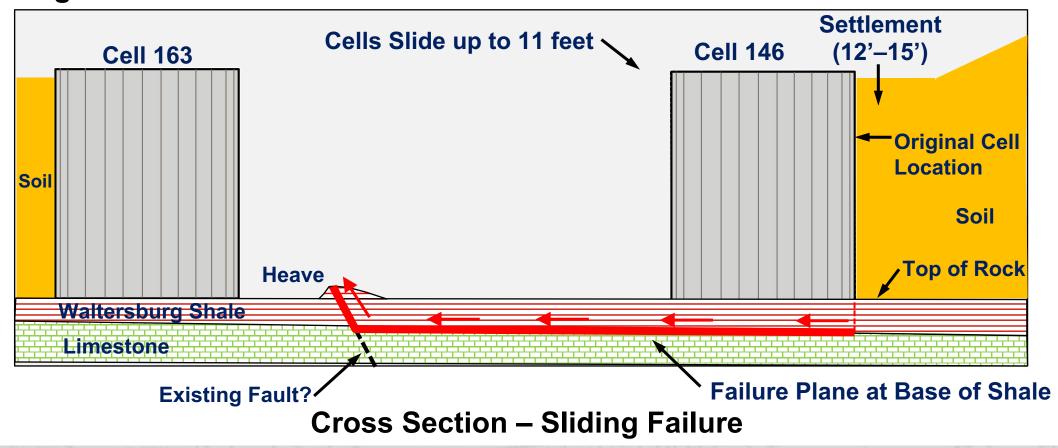


Plan View - Inner Ring of Cofferdam Cells (Tub Cells)



Cannelton Lock and Dam – Factors Leading to Failure

- Sliding along the base of the Mississippian-age Waltersburg Shale (clayey shale)
- Overestimated the sliding resistance at the base of the shale, with a backcalculated phi angle of 13° (possible clay seam)
- Reversal of stresses in shale may have reduced sliding resistance to residual strength

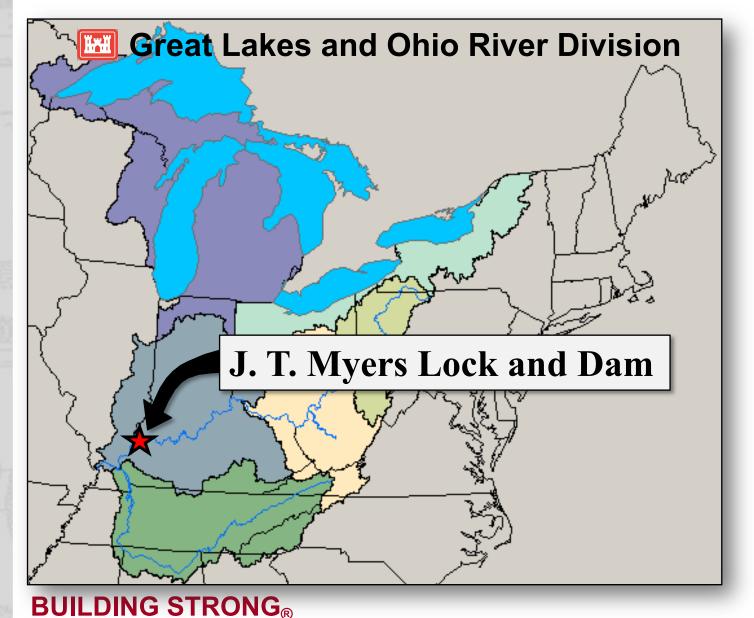


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John T Myers Lock and Dam (Uniontown) – General Info



and Taking Care of People!

- Located on the Ohio River at IN/KY boarder. Formerly known as Uniontown L&D
- Construction contractordesigned sheet-pile cell cofferdam
- Excavation for the dam pier foundations
- Cofferdam sliding failure on Feb 26, 1971.

John T Myers Lock and Dam – Cofferdam Prior to Failure

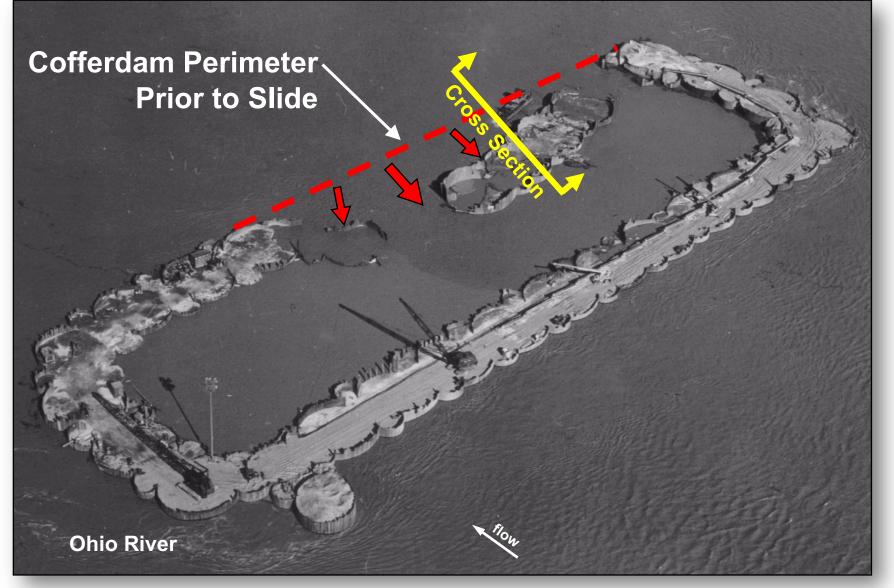


- Cofferdam sliding failure on Feb 26, 1971 during a period of high water, 10 days after dewatering.
- Started at 9:45am, movement over a period of <10 minutes
- 52 personnel were in the work area, all escaped harm.





John T Myers Lock and Dam – Cofferdam After Failure

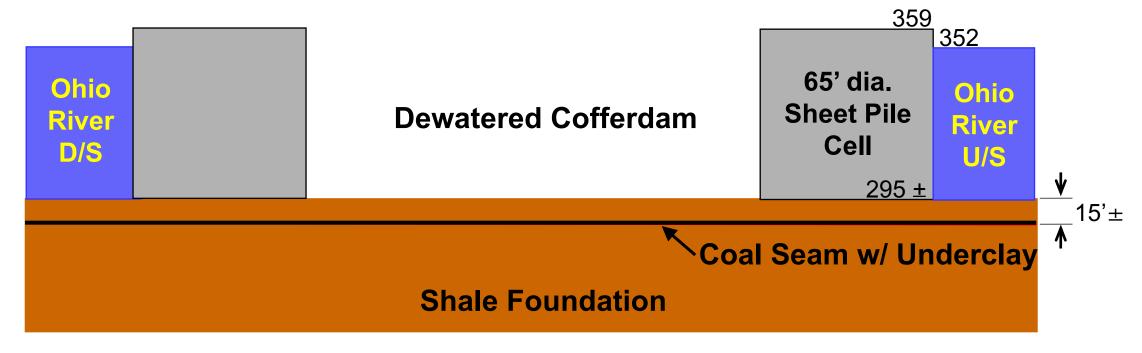


- One cell ruptured
- Five cells slid 32' to 71' upstream direction
- One cell slid and collapsed



John T Myers Lock and Dam – Planar Failure

- Sliding horizontally along a coal seam underclay, 15' below top of rock
- Pennsylvanian-age Sturgis Formation (previously Lisman Formation)

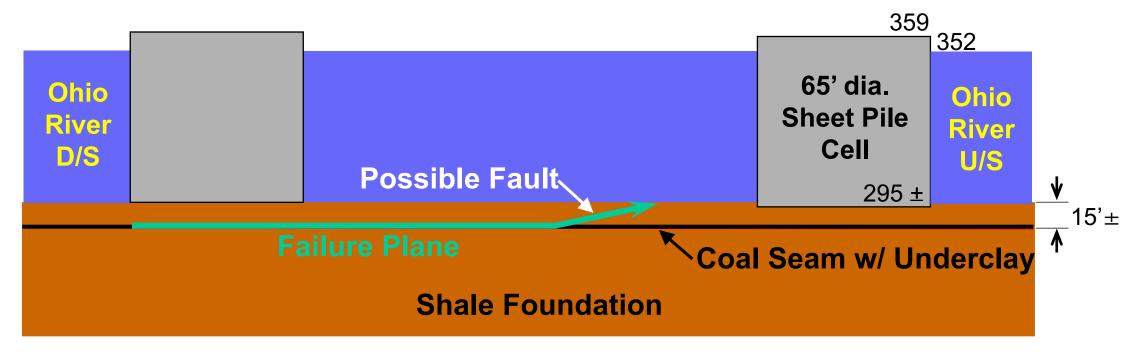


Cross Section – Cofferdam Failure – Deep Seated Sliding



John T Myers Lock and Dam – Factors Leading to Failure

- Overestimated sliding resistance of underclay
- Sliding resistance of passive wedge was reduced, due to possible fault planes



Cross Section – Cofferdam Failure – Deep Seated Sliding



Sliding Failures at Five Lock and Dam Projects

LESSONS LEARNED

- 1. Identify continuous <u>weak seams</u> in bedrock and establish <u>appropriate sliding parameter values</u> (φ angle).
- 2. Consider influence of <u>uplift pressures</u> on sliding stability.
- 3. Anticipate how construction activities could affect sliding stability.
- 4. Install <u>instrumentation</u> and monitor structure during critical loading conditions.
- 5. As a result, Corps of Engineers has designed all major cofferdams at Corps projects since 1970s

